Corrected Address: 1414 Henrietta Ave

PACIFICA ENGINEERING SERVICES

January 23, 2026

West Palm Beach Housing Authority
3700 Georgia Avenue
West Palm Beach, FL 33405

Attention: Mr. Michael McManaman
Email: mmcmanaman@wpbha.org
Phone: (561) 655-8530 X1106

Re: Report of Geotechnical Engineering Services
1400 Henrietta Avenue Development
1400 Henrietta Avenue
West Palm Beach, Florida 33401
PACIFICA Project No.: 320-25489

Dear Mr. McManaman:

Pacifica Engineering Services, LLC. (PACIFICA) has completed a geotechnical engineering study for the above-referenced project. The scope of geotechnical services was completed in general accordance with PACIFICA Proposal for Geotechnical Engineering Services Number 610-12019734 dated February 27, 2025. Authorization to proceed was provided via signature of the proposal on March 4, 2024.

PACIFICA appreciates the opportunity to provide geotechnical engineering services on this project and looks forward to an opportunity to participate in construction-related aspects of the development. If you have any questions or should additional information, be required, please do not hesitate to contact our office at (561) 419-8460.

Sincerely,

Pacifica Engineering Services
Florida Certification of Authorization License No. 32328

Marc Restrepo, E.I.
Staff Geotechnical Engineer

Andre Kniazeff
Digitally signed by Andre Kniazeff
DN: CN=Andre Kniazeff, ...
Date: 2026.01.23 15:53:11 -05'00'

ANDRE KNIAZEFF LICENSE No 81315 STATE OF FLORIDA PROFESSIONAL ENGINEER

Andre Kniazeff, P.E.
Chief Geotechnical Engineer
FL License No. 72242

CITY OF WEST PALM BEACH
BUILDING DIVISION
WPB
WEST PALM BEACH
PERMIT # 25120639-COM-BLDG
07/20/2026
OFFICIAL PERMIT RECORD

Corrected Address: 1414 Henrietta Ave

TABLE OF CONTENTS

APPENDIX:

1.0 GENERAL PROJECT INFORMATION

The project, 1400 Henrietta Avenue Development, is located at 1400 Henrietta Avenue in West Plam Beach, Florida. It is understood that plans for this project consist of the design and construction of a new one to two-story residential development with 18 units, a one-story clubhouse and associated paved/parking areas. A site vicinity map which shows the general location of the project site is located on Sheet 1 of the Appendix.

Structural loading was not provided at the time of this report; therefore, PACIFICA has assumed maximum columns and wall loads of 125 kips and 4 kips/linear foot, respectively. Similarly, site grading information was not provided, therefore PACIFICA has assumed existing site grades will be within 2 feet of final site grades.

If any of the information in this report is incorrect or has changed, please notify PACIFICA so that we may check the recommendations presented in this report. PACIFICA will not be held responsible if not given the opportunity to check the recommendations once final designs have been complete.

2.0 GEOTECHNICAL EXPLORATION

2.1 Geotechnical Boring

Five (5) Standard Penetration Test (SPT) geotechnical borings were performed to assess the subsurface conditions. The borings were advanced to a depth of 25 feet with respect to the site grades at the time of the geotechnical exploration. A boring location plan can be found on Sheet 2 of the Appendix.

The SPT borings were performed using a CME55 truck-mounted geotechnical drilling rig equipped with an automatic hammer utilizing mud rotary drilling techniques. The SPT samples were collected continuously in the upper 10 feet and at 5-foot intervals thereafter until final boring depth was reached.

After the samples were collected in the split barrel sampler they were bagged, labeled and transported back to the laboratory for description and limited testing. After the geotechnical borings were completed, they were backfilled using access auger cuttings and the ground surface was generally leveled.

2.2 Subsurface & Groundwater Conditions

The individual boring log included in the Appendix shows the various soil types and stratifications. The transition between soil strata may be gradual and not as definitive as it appears on the boring log. If the contractor cannot determine the soil strata during construction the geotechnical engineer should be consulted.

Groundwater was encountered at a depth of 6 feet at the time of the geotechnical exploration. The groundwater table may vary due to high/low tide fluctuations, rainfall, runoff, droughts or the infiltration rate of the soil and therefore the contractor should verify the groundwater table prior to construction.

2.3 Seasonal High Ground Water Table (SHGWT)

The groundwater depth was measured at the boring locations following the termination of drilling. Encountered groundwater depths measured in the boreholes are presented in the soil profiles in the Appendix. There may be differences in the groundwater table noted in the boreholes due to the variability in ground surface elevation between boring locations or because the groundwater levels in the boreholes did not stabilize given the short window they were measured in. The groundwater table may also vary due to rainfall, runoff, droughts, nearby water control structures, or the infiltration rate of the soil, and therefore the contractor should verify the groundwater table prior to construction.

Seasonal high groundwater levels are expected to be controlled by existing drainage features, the time of year, rainfall events, and water bodies present in the project vicinity. Based on the time of the geotechnical exploration, the groundwater levels have been recorded during the dry season during the month of March 2025. Therefore, the groundwater levels can be 18 to 24 inches higher during the peak of the wet season. In South Florida, the peak of the wet season is usually during the month of October when water levels are usually the highest. We recommend the project design be coordinated with adjacent projects and the SHGWT and Design High Water (DHW) be adjusted accordingly for a smooth transition in roadway and drainage profiles with the adjacent projects. For more accurate measurements of groundwater levels, we recommended the installation of monitoring wells and monitoring groundwater levels throughout the year.

3.0 GEOTECHNICAL CLASSIFICATION

The soil samples taken from the geotechnical boring was taken back to the laboratory and visually reviewed by a geotechnical engineer. The soil samples were classified using the Unified Soil Classification System (USCS) in general accordance with the American Society of Testing and Materials (ASTM) test designation D2487.

4.0 FOUNDATION RECOMMENDATIONS

4.1 Shallow Foundations Recommendations

The proposed one to two-story residential development may be supported on shallow spread footings/grade beams or monolithic reinforced slab-on-grade foundation system. Prior to placement of the shallow foundations properly compacted in-situ granular soils or structural fill soils should be placed and compacted within the footprint of each shallow foundation. Structural fill backfill classification and compaction requirements shall be followed and are outlined in Section 5.0 of this report.

Shallow foundations, or monolithic reinforced slab-on-grade systems, with an allowable bearing pressure of 2,500 pounds per square foot and placed at a minimum of 24 inches below finished floor elevation is recommended for the one to two-story residential development. To prevent against punching shear failure, the minimum footings widths for continuous and isolated footings should be 2 feet and 3 feet, respectively. For monolithic slab-on-grade foundation systems, grade beams supporting column loads shall have a minimum width of 2.5 feet and grade beams supporting wall loads shall have a minimum width of 1 foot.

Designers should use a friction factor of 0.55 for base shearing resistance and values calculated from this shearing resistance should be considered ultimate. Passive earth pressure resistance should be computed using an equivalent fluid pressure of 180 pounds per square foot per foot of depth, for granular materials. Additional soil values the designer may require for uplift design are the unit weight and submerged unit weight of the soil, 110 pcf and 48 pcf, respectively.

Settlement estimates were determined based on the assumed structural loads listed in Section 1.0 of this report and the allowable bearing capacity detailed in the beginning of this section. Please contact PACIFICA if the assumed loads are incorrect. PACIFICA is not responsible nor will be held liable if the assumed loads in this report are inaccurate. Settlement estimates were determined from empirical relationships and based on experience with similar projects. Based on the geotechnical exploration, the required compaction recommendations in this report and the assumed loads listed in Section 1.0, an estimated settlement of 1 inch or less, was calculated. Differential settlement between adjacent shallow foundations may be approximately ½ the total estimated settlement. Due to the elastic characteristics of granular soils the settlement should be observed as soon as the live loads are applied. These settlements are usually considered to be tolerable for the proposed structure. However, if more stringent settlements are required then PACIFICA should be contacted to revise the recommendations given in this report.

4.2 Ground Floor Slab Recommendations

Ground floor slabs can bear directly on top of compacted structural fill material that is compacted based on requirements in Section 5.0. A modulus of subgrade reaction value of 150 pounds per cubic inch (pci) may be used for design. The underside of the ground floor slab should be lined with a vapor barrier at least 6 mil thick. A friction factor of 0.21 (ultimate) can be used for the soil to vapor barrier interface.

5.0 SITE PREPARATION RECOMMENDATIONS

5.1 General Site Preparation

The results of the field investigation and experience from similar projects have yielded the following recommendations detailed in steps. These steps should be conducted by an experienced contractor adhering to current regulatory standards. It should be noted that these steps should be performed with care to not damage any adjacent structures or any underground utilities.

1) Prior to construction activities on-site underground utilities should be identified and marked in the field. If any of the existing utility lines are in conflict with the proposed construction, then plans should be made to relocate these lines. If the utilities discovered are abandoned or out of service, they should be removed.
2) Topsoil, asphalt, concrete, unsuitable material or any other debris should be stripped to expose in-situ soils. If any old foundations from previous structures are encountered, then they should be removed in their entirety. If deep foundations are encountered the geotechnical engineer should be retained to assess the impact on the proposed structures and to make recommendations on mitigation.
3) The exposed on-site soils in the building(s) footprint should be properly compacted per the recommendations in the section entitled Compaction Recommendations. The compaction should extend 5 feet beyond the perimeter of the building.

4) Any unsuitable material or debris encountered should be removed and replaced with structural fill.
5) The contractor should prepare for heavy excavation activities as limerock or sand with limestone was encountered at relatively shallow depths. If limestone is encountered, it may require special tools/buckets in order to break the material up.
6) Any fill needed to bring up the site to the proper elevation, including any reference to structural fill in this report, should adhere to the recommendations given the section entitled Structural Fill Soils.
7) A representative of the geotechnical engineer should be present and properly document these activities.
8) Any other geotechnical related questions should be directed to the geotechnical engineer of record.

The contractor should adhere and be aware of all OSHA and any regulatory standards during construction activities. The contractor is responsible and held solely liable if these standards are not upheld.

5.2 Dewatering Recommendations

Dewatering may be required on this site if deeper excavations are warranted. Groundwater may be pumped out using pumps or other processes to at least 2 feet below and compaction activities. Well point systems may be used if deeper excavations are required for proper and safe construction. A specialty contractor should design these systems and adhere to any regulatory standards.

5.3 Compaction Recommendations

Once initial site clearing has been performed, the exposed in-situ soils in the building(s) footprint should be properly compacted until the surface is firm and unyielding. The compaction should extend 5 feet beyond the perimeter of the building(s). Care should be taken when compacting adjacent to existing structures. A self-propelled vibratory roller should be used to compact the exposed in-situ soils. The proof rolling should be observed by PACIFICA to identify and mitigate any weak subgrade conditions.

If any locations of in-situ soils overly deflect under the weight of the roller then the soils should be removed to a depth of 24 inches and replaced with properly compacted structural fill materials. The structural fill soils should be compacted to 98% of the Modified Proctor maximum dry density per ASTM D1557. Wetting of the subgrade sols may be used in order to achieve proper compaction.

5.4 Structural Fill Soils

Structural fill soils should be inorganic and consist of granular material containing less than 12 percent passing the U.S. Standard No. 200 mesh sieve, a maximum particle size of 3 inches and have a Unified Soil Classification System (USCS) designation of GP, GW, GP-GM, GW-GM, SP, SW, SP-SM or SW-SM. The structural fill material may be composed of either clean sands and/or limerock. The use of “Cyclone Sand” is not permitted.

Density tests should be performed by a qualified technician working under the supervision of the geotechnical engineer and be in accordance with the appropriate ASTM standards. The representative of the geotechnical engineer should be present and agree with the placement and compaction of all structural fill materials.

Loose lifts not exceeding 12 inches should be performed on all structural fill materials. The lifts should be compacted to 98% of the Modified Proctor (ASTM D1557). If a small vibratory plate or roller is used, then loose lifts should not exceed 8 inches.

5.5 Shallow Foundation Construction

Based on the results of the geotechnical exploration, the following recommendations apply for the soils beneath the building(s) foundations. In-situ or imported structural fill soils at the base of the footing excavations should be compacted to at least 98% of the Modified Proctor (ASTM D1557). If any locations of in-situ or imported structural fill soils overly deflect under the weight of the compactor/roller then the soils should be removed to a depth of 24 inches and replaced with properly compacted structural fill materials.

6.0 PAVEMENT RECOMMENDATIONS

Both flexible and rigid pavement sections may be used for this project. The pavement sections require a sub-base consisting of one or multiple layers. Traffic loading has been assumed for this project and the following recommendations have been made in the following table. Once final traffic loads and estimates have been made a civil engineer should review these general pavement recommendations given in Table 1 to check the validity.

TABLE 1: GENERAL PAVEMENT SECTION RECOMMENDATIONS

Type of Pavements	Sections/Layers	Section/Layer Thickness (in)
Flexible	Florida DOT Asphalt Type S	2.0
	Crushed limerock compacted to 98% of the Modified Proctor. A minimum LBR of 100 is required.	8.0
	Stabilized sub-base fill compacted to 98% of the Modified Proctor. A minimum LBR of 40 is required.	12.0
Rigid	Florida DOT Portland Cement Concrete	6.0
	Stabilized sub-base fill compacted to 98% of the Modified Proctor. A minimum LBR of 40 is required.	12.0

Notes:

1. Sub-base fill materials should meet the requirements presented in the latest revisions of the FDOT "Specifications for Road and Bridge Construction", Section 911.

The above pavement sections and thicknesses were based on assumed values taken from similar projects. A Civil Engineer should perform a design once traffic loading and estimates are finalized. The design pavement section shall meet minimum city/county requirements.

In any areas where dumpsters or heavy equipment are to be stored for extended periods of time, it is recommended that the rigid pavement section be utilized. Periodic maintenance should be expected for the lifetime of these pavement systems.

Corrected Address: 1414 Henrietta Ave

7.0 REPORT LIMITATIONS

Our geotechnical engineering services have been performed, findings obtained, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices at the time of this report. This company is not responsible for the conclusions, opinions or recommendations made by others based on this data. No other warranties are implied or expressed.

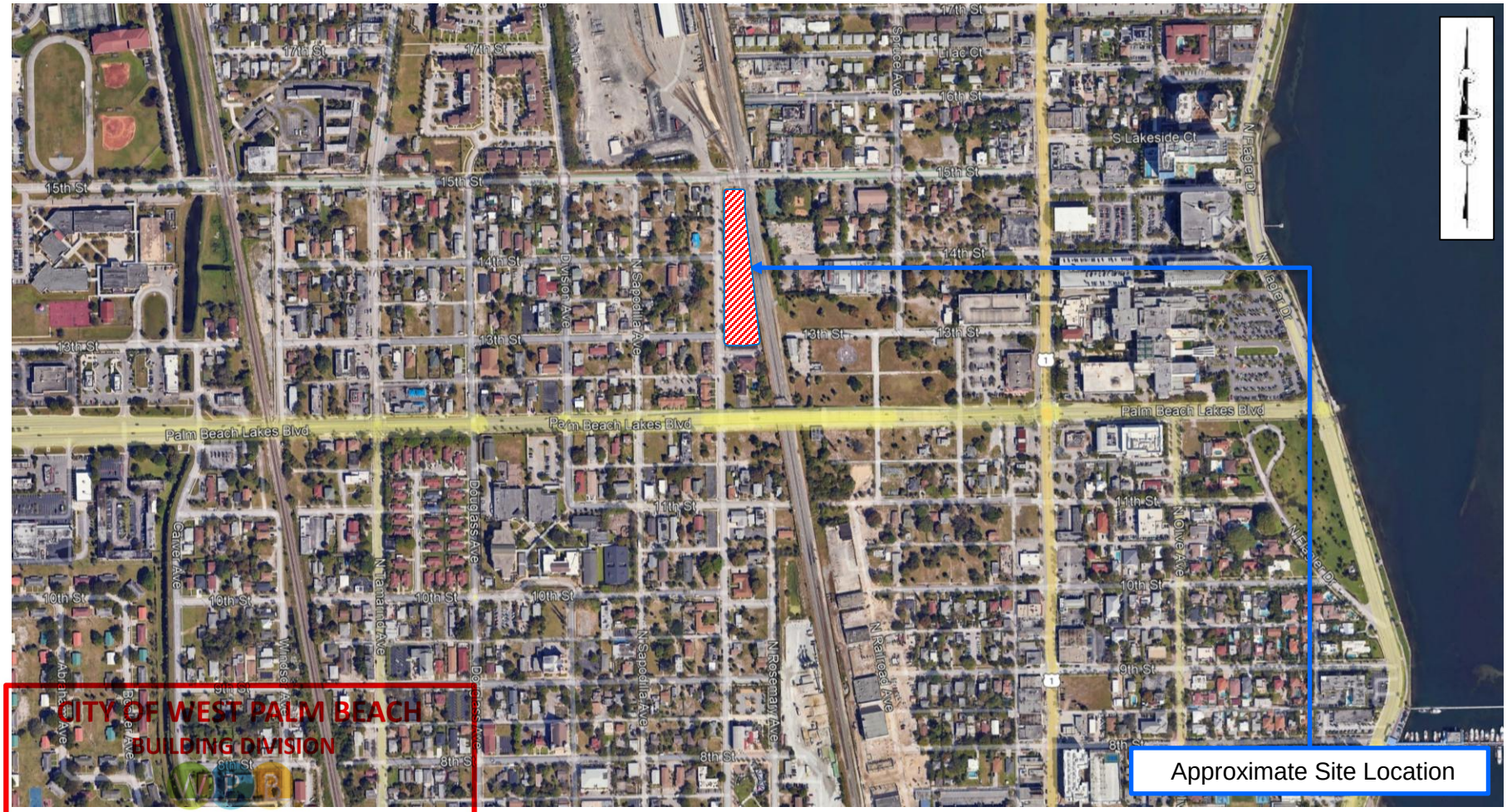
After the plans and specifications are complete, PACIFICA should be provided the opportunity to review the final design and specifications, in order to verify that the earthwork and foundation recommendations are properly interpreted and implemented. At that time, it may be necessary to submit supplemental recommendations. If PACIFICA is not afforded the opportunity to participate in construction related aspects of foundation installation as recommended in this report, we can accept no responsibility for the interpretation of our recommendations made in this report or for foundation performance.

The scope of investigation was intended to evaluate soil conditions within the influence of the proposed foundations. The analyses and recommendations submitted in this report are based upon the data obtained from the soil borings performed at the locations indicated. If any subsoil variations become evident during the course of this project, a re-evaluation of the recommendations contained in this report will be necessary after we have had an opportunity to observe the characteristics of the conditions encountered. The applicability of the report should also be reviewed in the event significant changes occur in the design, nature or location of the proposed structures. The scope of our services did not include an environmental assessment for the presence or absence of hazardous or toxic materials in the soil and groundwater. Any statements in this report regarding odors, staining of soils, or other unusual conditions observed are strictly for the information of our client.

This report has been prepared for the exclusive use of the West Palm Beach Housing Authority and their design consultants for the construction of the one to two-story residential 1400 Henrietta Development located at 1400 Henrietta Avenue in West Palm Beach, Florida.

Corrected Address: 1414 Henrietta Ave

SITE VICINITY MAP



CITY OF WEST PALM BEACH
BUILDING DIVISION
WEST PALM BEACH
PERMIT #25120269 COMBLDG
07/20/2026
OFFICIAL PERMIT RECORD

GEOTECHNICAL ENGINEERING SERVICES
1400 Henrietta Ave Development
1400 Henrietta Avenue, West Palm Beach, Florida

DATE: 03/20/2025

DRAWN: MR

SHEET NO.: 1

PACIFICA PROJ. NO: 320-25489

CHKD: PM

PACIFICA ENGINEERING SERVICES

Corrected Address: 1414 Henrietta Ave

BORING LOCATION PLAN - AERIAL

B-# Approximate SPT Boring Location

CITY OF WEST PALM BEACH
BUILDING DIVISION
WEST PALM BEACH
PERMIT # 25120039 COMBLDG
07/20/2026
OFFICIAL PERMIT RECORD

GEOTECHNICAL ENGINEERING SERVICES
1400 Henrietta Ave Development
1400 Henrietta Avenue, West Palm Beach, Florida

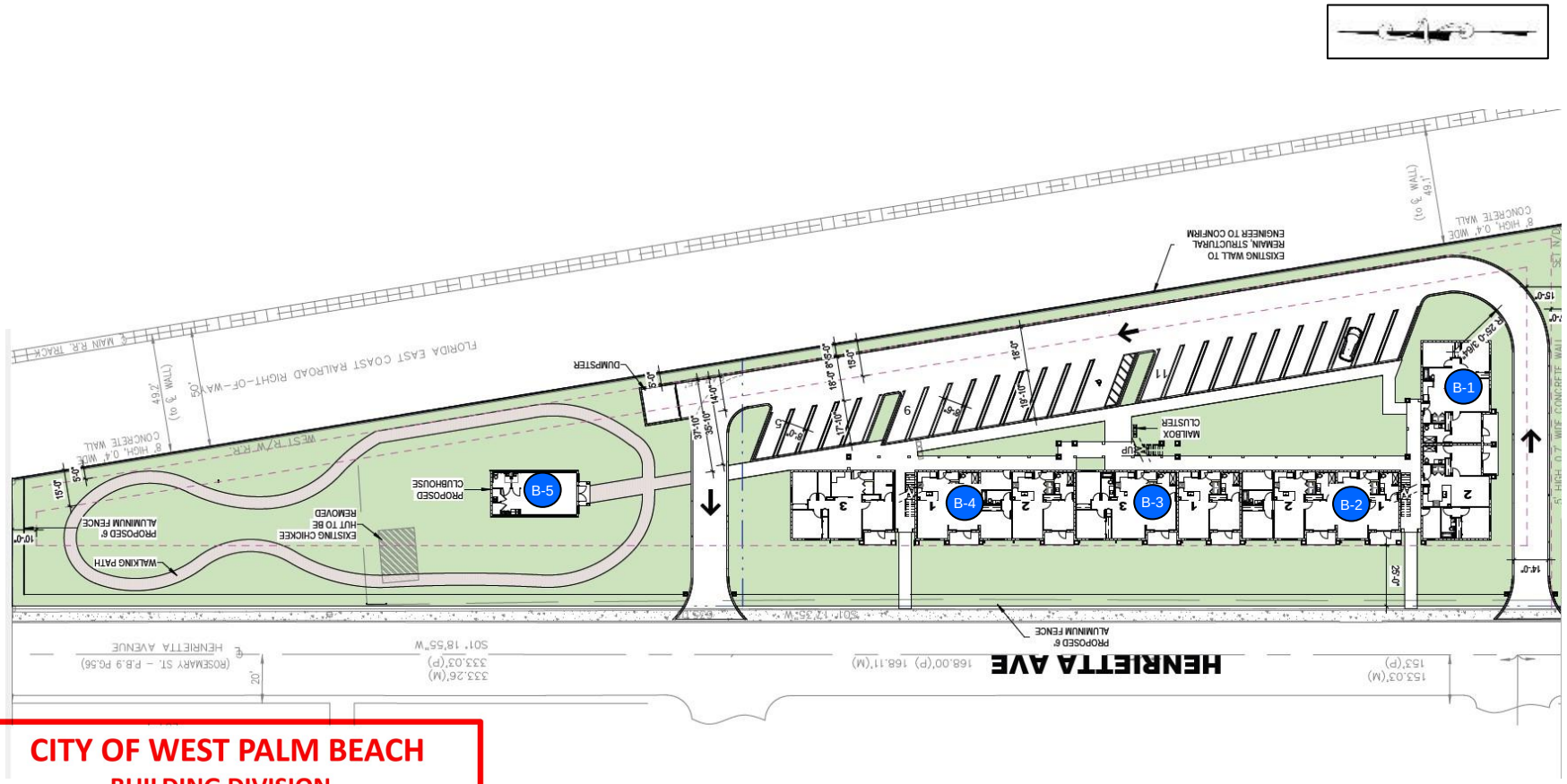
DATE: 03/20/2025
DRAWN: MR
CHKD: PM

SHEET NO.: 2 PACIFICA PROJ. NO: 320-25489

PACIFICA ENGINEERING SERVICES

Corrected Address: 1414 Henrietta Ave

BORING LOCATION PLAN - SITE



B-# Approximate SPT Boring Location

CITY OF WEST PALM BEACH
BUILDING DIVISION
WEST PALM BEACH

PERMIT #25120469 COMBLDG
07/20/2026
OFFICIAL PERMIT RECORD

GEOTECHNICAL ENGINEERING SERVICES
1400 Henrietta Ave Development
1400 Henrietta Avenue, West Palm Beach, Florida

SHEET NO.: 3 PACIFICA PROJ. NO: 320-25489

DATE: 03/20/2025
DRAWN: MR
CHKD: PM

PACIFICA ENGINEERING SERVICES

Corrected Address: 1414 Henrietta Ave

PACIFICA ENGINEERING SERVICES

Test Boring Log

Client	**West Palm Beach Housing Authority**	Boring No.	**B-1**
Project	1400 Henrietta Ave Development	Date Started	3/21/2024
Boring Location	See Boring Location Plan	Date Completed	3/21/2024
Elev. Ref.	N/A	Project No.	320-25489
Remarks	The stratification lines represent approximate boundaries.	Sheet No.	1 of 1
	The transition may be gradual.	Ground Water Depth	feet

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value
	0.0		3" Topsoil / Gray to Brown Fine SAND	1			9-8-6-4			14
	2.0			2			6-7-5-3			12
▼	4.0			3			4-6-9-8			15
	6.0			4			5-4-5-3			9
	8.0			5			3-4-3-2			7
	10.0									
	12.0			6			6-8-9-5			17
	14.0									
	16.0			7			3-4-7-6			11
	18.0									
	20.0			8			5-8-9-8			17
	22.0									
	24.0									
	26.0		Boring Terminated at 25.0 feet							

CITY OF WEST PALM BEACH
BUILDING DIVISION
WEST PALM BEACH
PERMIT # 25120639-COM BLDG
07/20/2026
OFFICIAL PERMIT RECORD

General Notes

Driller:	W.S.
Hammer Type:	Automatic
Rig Type	CME55
Drilling Method	SPT

PACIFICA ENGINEERING SERVICES
601 North Congress Avenue - Suite 303 | Delray Beach, Florida 33445
(561) 419-8460

SPT Split Spoon Sampler
Groundwater at Time of Drilling

Corrected Address: 1414 Henrietta Ave

PACIFICA ENGINEERING SERVICES

Test Boring Log

Client	**West Palm Beach Housing Authority**	Boring No.	**B-2**
Project	1400 Henrietta Ave Development	Date Started	3/21/2024
Boring Location	See Boring Location Plan	Date Completed	3/21/2024
Elev. Ref.	N/A	Project No.	320-25489
Remarks	The stratification lines represent approximate boundaries.	Sheet No.	1 of 1
	The transition may be gradual.	Ground Water Depth	6.0 feet

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value
	0.0		3" Topsoil / Gray to Brown Fine SAND	1			10-6-5-2			7
	2.0			2			4-2-3-2			5
▼	4.0			3			1-2-3-4			5
	6.0			4			5-7-6-4			13
	8.0			5			6-8-3-1			11
	10.0									
	12.0			6			5-8-7-4			15
	14.0									
	16.0			7			3-5-8-4			13
	18.0									
	20.0			8			8-6-4-7			10
	22.0									
	24.0									
	26.0		Boring Terminated at 25.0 feet							

CITY OF WEST PALM BEACH
BUILDING DIVISION
WEST PALM BEACH
PERMIT # 25120639-COM BLDG
07/20/2026
OFFICIAL PERMIT RECORD

General Notes

Driller:	W.S.
Hammer Type:	Automatic
Rig Type	CME55
Drilling Method	SPT

PACIFICA ENGINEERING SERVICES
601 North Congress Avenue - Suite 303 | Delray Beach, Florida 33445
(561) 419-8460

SPT Split Spoon Sampler
Groundwater at Time of Drilling

Corrected Address: 1414 Henrietta Ave

PACIFICA ENGINEERING SERVICES

Test Boring Log

Client	**West Palm Beach Housing Authority**	Boring No.	**B-3**
Project	1400 Henrietta Ave Development	Date Started	3/20/2024
Boring Location	See Boring Location Plan	Date Completed	3/20/2024
Elev. Ref.	N/A	Project No.	320-25489
Remarks	The stratification lines represent approximate boundaries.	Sheet No.	1 of 1
	The transition may be gradual.	Ground Water Depth	6.0 feet

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value
	0.0									
	2.0			1			9-7-6-4			13
	4.0			2			3-2-1-2			3
▼	6.0			3			4-5-7-4			12
	8.0			4			8-6-4-1			10
	10.0			5			7-4-3-1			7
	12.0									
	14.0		3" Topsoil / Gray to Brown Fine SAND	6			3-2-2-4			4
	16.0									
	18.0									
	20.0			7			5-3-4-6			7
	22.0									
	24.0			8			7-10-12-9			22
	26.0		Boring Terminated at 25.0 feet							

CITY OF WEST PALM BEACH
BUILDING DIVISION
WEST PALM BEACH
PERMIT # 25120639-COM BLDG
07/20/2026
OFFICIAL PERMIT RECORD

General Notes

Driller:	W.S.
Hammer Type:	Automatic
Rig Type	CME55
Drilling Method	SPT

PACIFICA ENGINEERING SERVICES
601 North Congress Avenue - Suite 303 | Delray Beach, Florida 33445
(561) 419-8460

▩ SPT Split Spoon Sampler
▼ Groundwater at Time of Drilling

Corrected Address: 1414 Henrietta Ave

PACIFICA ENGINEERING SERVICES

Test Boring Log

Client	**West Palm Beach Housing Authority**	Boring No.	**B-4**
Project	1400 Henrietta Ave Development	Date Started	3/20/2024
Boring Location	See Boring Location Plan	Date Completed	3/20/2024
Elev. Ref.	N/A	Project No.	320-25489
Remarks	The stratification lines represent approximate boundaries.	Sheet No.	1 of 1
	The transition may be gradual.	Ground Water Depth	6.0 feet

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value
	0.0									
	2.0			1			4-3-4-2			7
	4.0			2			3-3-5-3			8
▼	6.0			3			5-8-6-4			14
	8.0			4			4-6-3-3			9
	10.0			5			7-4-5-6			9
	12.0									
	14.0		3" Topsoil / Gray to Brown Fine SAND	6			4-6-7-9			13
	16.0									
	18.0									
	20.0			7			3-2-1-1			3
	22.0									
	24.0			8			6-9-7-10			16
	26.0		Boring Terminated at 25.0 feet							

CITY OF WEST PALM BEACH
BUILDING DIVISION
WEST PALM BEACH
PERMIT # 25120639-COM BLDG
07/20/2026
OFFICIAL PERMIT RECORD

General Notes

Driller:	W.S.
Hammer Type:	Automatic
Rig Type	CME55
Drilling Method	SPT

PACIFICA ENGINEERING SERVICES
601 North Congress Avenue - Suite 303 | Delray Beach, Florida 33445
(561) 419-8460

▀ SPT Split Spoon Sampler
▼ Groundwater at Time of Drilling

Corrected Address: 1414 Henrietta Ave

PACIFICA ENGINEERING SERVICES

Test Boring Log

Client	**West Palm Beach Housing Authority**	Boring No.	**B-5**
Project	1400 Henrietta Ave Development	Date Started	3/20/2024
Boring Location	See Boring Location Plan	Date Completed	3/20/2024
Elev. Ref.	N/A	Project No.	320-25489
Remarks	The stratification lines represent approximate boundaries.	Sheet No.	1 of 1
	The transition may be gradual.	Ground Water Depth	6.0 feet

ELEV. (ft)	Depth (ft)	Graphic Log	DESCRIPTION OF MATERIALS	SAMPLE No.	Type	USCS Classification	Blows	REC-OVERY	REC %	SPT N. Value
	0.0		3" Topsoil / Gray to Brown Fine SAND	1			6-4-8-3			12
	2.0			2			2-4-2-3			6
▼	4.0			3			5-7-6-4			13
	6.0			4			4-2-3-5			5
	8.0			5			5-4-6-2			10
	10.0									
	12.0			6			8-7-4-3			11
	14.0									
	16.0			7			6-9-7-4			16
	18.0									
	20.0			8			8-6-9-7			15
	22.0									
	24.0									
	26.0		Boring Terminated at 25.0 feet							

CITY OF WEST PALM BEACH
BUILDING DIVISION
WEST PALM BEACH
PERMIT # 25120639-COM BLDG
07/20/2026
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General Notes

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Hammer Type:	Automatic
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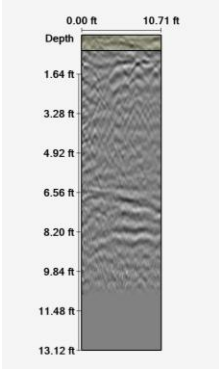
601 North Congress Avenue - Suite 303 | Delray Beach, Florida 33445
(561) 419-8460

SPT Split Spoon Sampler
Groundwater at Time of Drilling

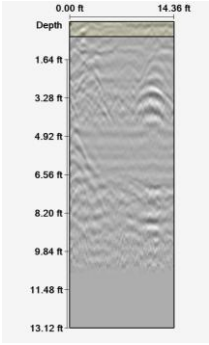
Corrected Address: 1414 Henrietta Ave

GROUND PENETRATING RADAR (GPR) SCANNING DOCUMENTATION

B-1 West to East Scan



B-1 South to North Scan



GEOTECHNICAL ENGINEERING SERVICES
1400 Henrietta Ave Development
1400 Henrietta Avenue, West Palm Beach, Florida

DATE: 03/20/2025

DRAWN: MR

CHKD: PM

SHEET NO.: 1

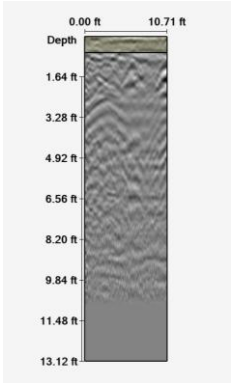
PACIFICA PROJ. NO: 320-25489

PACIFICA ENGINEERING SERVICES

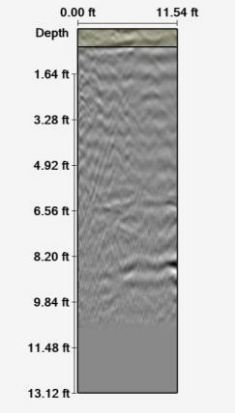
Corrected Address: 1414 Henrietta Ave

GROUND PENETRATING RADAR (GPR) SCANNING DOCUMENTATION

B-2 West to East Scan



B-2 South to North Scan



GEOTECHNICAL ENGINEERING SERVICES
1400 Henrietta Ave Development
1400 Henrietta Avenue, West Palm Beach, Florida

DATE: 03/20/2025
DRAWN: MR
CHKD: PM

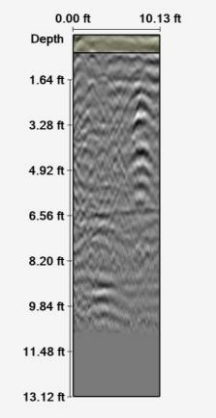
SHEET NO.: 2
PACIFICA PROJ. NO: 320-25489

PACIFICA ENGINEERING SERVICES

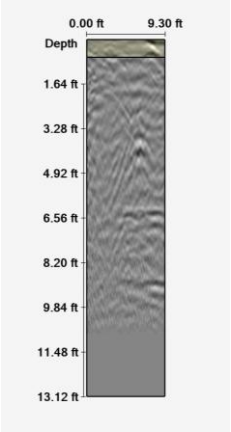
Corrected Address: 1414 Henrietta Ave

GROUND PENETRATING RADAR (GPR) SCANNING DOCUMENTATION

B-3 West to East Scan



B-3 South to North Scan



GEOTECHNICAL ENGINEERING SERVICES
1400 Henrietta Ave Development
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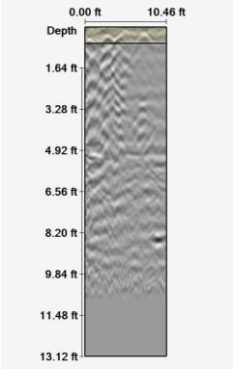
SHEET NO.: 3 PACIFICA PROJ. NO: 320-25489

PACIFICA ENGINEERING SERVICES

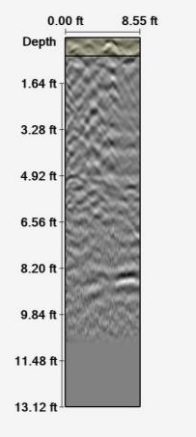
Corrected Address: 1414 Henrietta Ave

GROUND PENETRATING RADAR (GPR) SCANNING DOCUMENTATION

B-4 West to East Scan



B-4 South to North Scan



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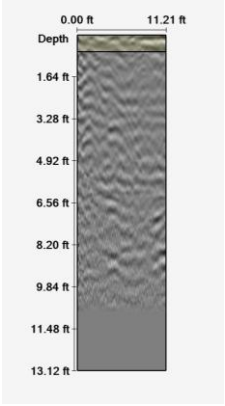
PACIFICA ENGINEERING SERVICES

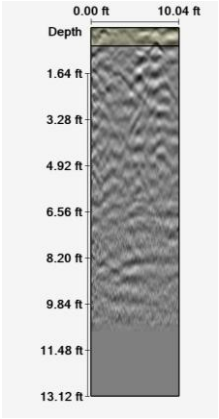
Corrected Address: 1414 Henrietta Ave

GROUND PENETRATING RADAR (GPR) SCANNING DOCUMENTATION

B-5 West to East Scan



B-5 South to North Scan



GEOTECHNICAL ENGINEERING SERVICES
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SHEET NO.: 5 PACIFICA PROJ. NO: 320-25489

PACIFICA ENGINEERING SERVICES